# PRELIMINARY DRAINAGE REPORT

1141 Chemawa Road Keizer, Oregon

Prepared For: Backus Investments, LLC 2415 Perkins Street Salem, Oregon 97302

June 30, 2023





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### **PROJECT DESCRIPTION**

The applicant is applying to partition a parcel into two lots and rezone one of the lots to develop into a multi-family development. The applicant intends to construct a 20-unit apartment complex that will disturb approximately 0.93-acres. The location of the site is 1141 Chemawa Road. The parcel of land to be developed is a portion of Tax Lot 2100 of Marion County Assessor's Map 07 3W 03BA. Supporting maps for the site are in Appendix A of this report.



#### **Project Site**

The development will consist of two apartment buildings containing 20 units and will provide driveway access and a parking lot. The proposed development will be connected to public water and public sewer. Stormwater conveyance and detention will be designed per the current City of Keizer's Public Works Design Standards. The on-site storm facility will be a Combination/Infiltration facility. Runoff from newly created impervious surfaces will be conveyed to the system that has been designed to infiltrate all runoff generated from the site.

#### **EXISTING CONDITIONS**

The 0.93-acre site is rectangular in shape. Surface vegetation consists of grass, minimal trees, and accessory buildings. There are no identified wetlands, sensitive areas or waterways located on the property. The topographical high point is located along the southerly property line. Drainage from this high point flows northerly. The relief is approximately 2-feet, and the property does appear to be hydrologically isolated. Appendix A contains a map of existing and proposed conditions.

The abutting properties are zoned residential single family and commercial general with public improvements. Appendix A contains a map of existing and proposed conditions.

The Soil Conservation Service Soil Survey of Marion County identifies the predominate soils on the site as a Cloquato silt loam (map unit Cm). The soil is in the hydrologic soil group B. Appendix B contains the NRCS soil survey for the site.

### **DEVELOPED CONDITIONS**

Stormwater runoff from the site will be conveyed and disposed of via a combination facility. A site map is in Appendix A showing the developed areas. Infiltration testing was performed at the site by Branch Engineering Inc. to determine percolation rates for the development. Testing results at the facility location indicate average percolation rates of 8.9 inches per hour. Using a safety factor of two, a percolation rate of 4.5 inches per hour will be used for design purposes. Appendix B contains the testing analysis and results.

Stormwater runoff from the apartment development will be conveyed to the combination facility designed using the current City of Keizer design standards and the 2020 Portland Stormwater Management Manual for guidance. The facility will have a capacity to infiltrate and detain all runoff generated by all storm events up to the 100-year event for the newly created impervious surfaces.

#### **STORMWATER ANALYSIS**

Stormwater quantity within the site is proposed to be handled via a 40' X 40' X 3' combinations facility. Runoff will be routed to the facility that stores and infiltrates runoff for all storm events that includes the 100-year event. An infiltration rate of 4.5 inches per hour was used in the analysis.

Post developed flow and infiltration rates were calculated using HydroCAD 10.20. Since complete infiltration is being used for the site, Table 1 below only lists the 24-hour rainfall depth for the 100-year storm event. Because of the proximity to the City of Keizer, City of Salem rainfall depths were used in the analysis.

Table 1					
24-hour					
Storm Event	Rainfall Depth				
	(in)				
100	4.40				

For the post-developed conditions, a time of concentration of 5 minutes was assumed. The on-site postdeveloped area was classified as "Impervious Area", HSG B with a curve number (CN) of 98 and ">75% Grass cover", Good, HSG B with a CN of 61. The weighted CN is 84. A basin map is in Appendix A. A Type 1A rainfall distribution was used with the above rainfall depth. The Santa Barbara Unit Hydrograph method was used to generate the hydrographs.

Table 2 below identifies the pertinent data for the 100-year storm event on the system. The calculations are incorporated in the HydroCAD output located in Appendix C.

Storm Event	Required Storage (cuft.)	Storage Provided (cuft.)	Infiltration Rate (inches/hrs)	Release Rate (cfs)		
100-year	1,068	3,900	4.5	0.00		
(Apartment Site)						

Table 2

In summary, as noted above, the systems as designed will retain and dispose of the runoff from the project and exceeds the City of Keizer design standards. The systems are compliant with the City's WPCF permit with Oregon DEQ by maintaining a separation distance of 2.5 feet from the average water table depth. The average water depth is approximately 25-feet and was obtained from the City of Keizer's Groundwater Protection Model. Attached in Appendix D is the City's correspondence email.

#### **OPERATION & MAINTENANCE**

The drainage system for the site is a unique and critical system. It is a complete infiltration system, which allows the water to collect and percolate into the subsurface. The only connection to a City of Keizer storm drainage system is the emergency overflow. All the stormwater generated onsite is to be disposed of through this infiltration system and released directly into the ground.

Since this facility works as a closed system, all dirt and sediment that enters onto the site (by way of wind, car tires, etc.) will remain within the stormwater system, unless it is maintained, and the sediment material is physically removed from the site.

A system as unique as this one must be properly maintained. As sediment and fine particles fill the system, the native soil will develop a sediment "cap" which will limit or completely stop the stormwater runoff from infiltrating into the native soil. This type of system failure can create conditions that require the entire system to be completely removed and replaced.

This manual describes the maintenance activities required to prolong the life of the facility and reduce the occurrence of a system failure that requires replacement.

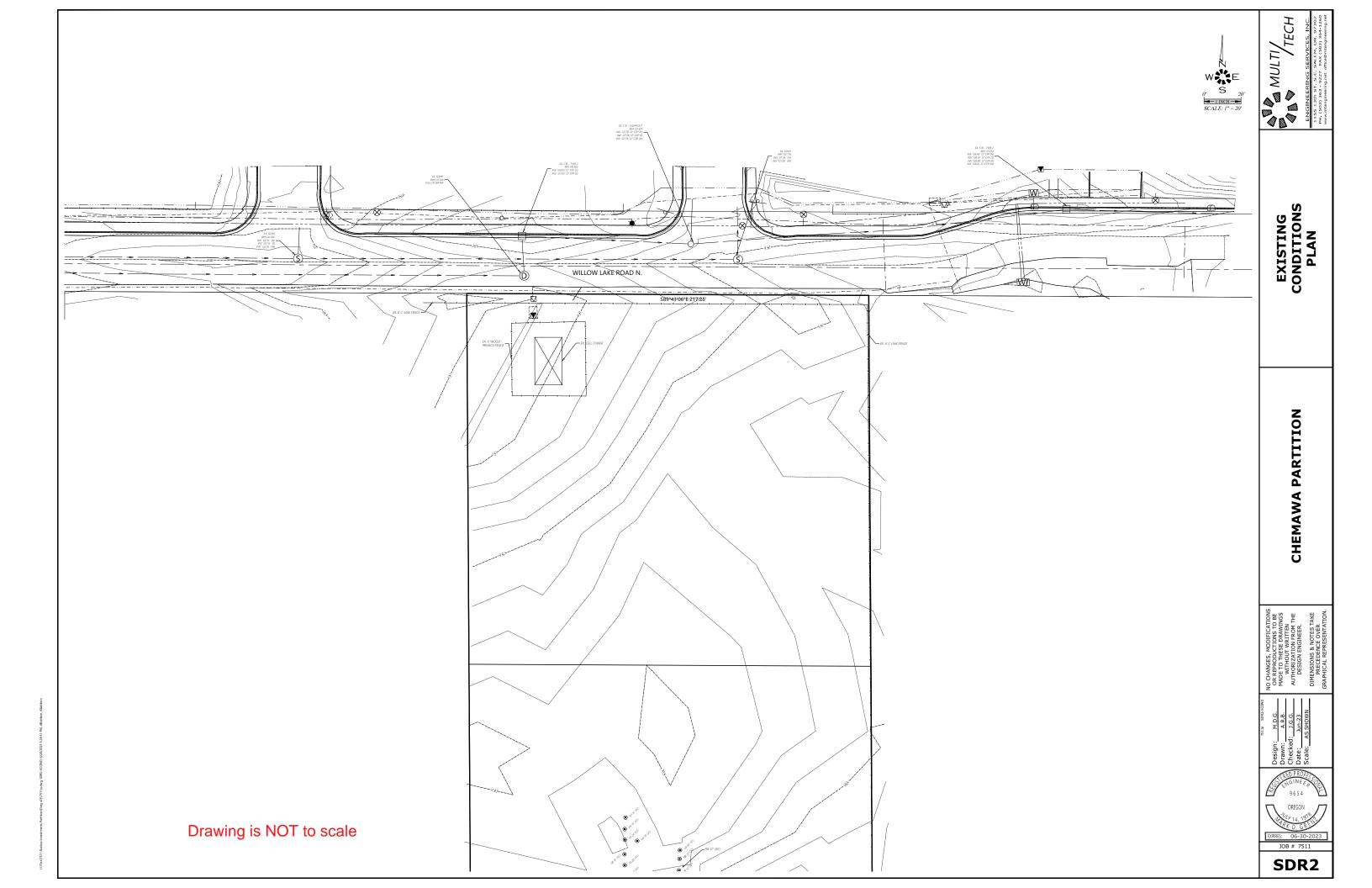
Operation and maintenance of the stormwater facility will be the responsibility of the property owner. O&M specifications with a checklist can be seen in Appendix D.

## CONCLUSION

Based on the presented information, the proposed design meets the water quantity standards. If there are any questions regarding this analysis or the design, please contact Matthew Hendrick at Multi/Tech Engineering by phone at (503) 363-9227 or via e-mail at <u>mhendrick@mtengineering.net</u>.

Appendix A





Appendix B



March 31, 2023

Jeremy Grenz Multi-Tech Engineering Inc 1155 13<sup>th</sup> Street SE Salem, Oregon

RE: SITE INFILTRATION TEST RESULTS CHEMAWA PARTITION APARTMENTS TAX MAP 06-3W-34CD TAX LOT 2100 KEIZER, OREGON BRANCH ENGINEERING INC. PROJECT NO. 23-107

At the request of the Multi-Tech Engineering Inc., Branch Engineering Inc (BEI). Has conducted site infiltration testing for the design of proposed stormwater systems at the subject site. The purpose of this investigation was to evaluate the infiltration rate of native soils that will be exposed to stormwater runoff from the proposed development. The results presented herein are for initial design and should be verified by the design engineer of record (EOR) at the time of construction. The scope of our services has included a review of the proposed site development, observations, logging, of three test pit excavations which were utilized for performing encased falling head infiltration tests at the approximate locations shown on the attached Figure-1.

#### Site Soils

The soils observed in the site test pit excavations were visually classified using the American Society of Testing and Materials (ASTM) Method D-2488) and logged by BEI staff (see attached infiltration soil logs). Observed soil conditions consisted of approximately 8- to 12-inches of brown silt topsoil, followed by a maximum of 34-inches of brown silty sand (SM), underlain by brown silt with little sand (ML). The deposition of the coarser grained material over the underlying fine-grained materials is likely a result of a large flood event, or series of flood events as faint lenses of silt were present within the larger silty sand horizon.

Nearby Oregon Water Resources Department (OWRD) well logs have been included with this report to establish the soil and groundwater conditions that are likely present beneath the site. Surficial soil conditions were similar to those observed in our onsite exploration with gravel being encountered at approximately 23-feet BGS in a boring completely a few hundred feet to the south. The NRCS Web Soil Survey of the Marion County Area shows that site soils are mapped as Cloquato silt loam, which forms on flood plains, is composed primarily of silt loam with a parent material of alluvium, and is considered well drained.

#### Ground Water

BEI staff observed the test pit excavations for the infiltration tests, and the geotechnical test pit excavations which advanced to a maximum depth of 10-feet BGS for deeper soil observations. We did not encounter the regional groundwater table; however, a very slow (<0.25 gal/min estimated) seep was observed at 8-feet BGS in Test Pit-3. The attached well logs indicate that groundwater was measured between 26- and 27-feet BGS at the well's locations. The elevation of the groundwater table

in the vicinity of the subject site is anticipated to be coincident with the Willamette River (approximately 25-feet lower in elevation than the site).

#### Infiltration Data Analysis

Site infiltration testing was conducted on March 8, 2023 in accordance with the 2014 City of Salem Administrative Rules. The soil is assumed to be laterally homogeneous and sidewall infiltration is negligible as a 6-inch diameter, open-ended, plastic standpipe was used for containment of the water column. Water was added to the pipe to pre-saturate the soil prior to testing. Infiltration testing commenced over three successive trials with water being added and the height of the water column being recorded over time. The measured infiltration rate is shown below in Table 1 (no factor of safety applied), with the field data presented in the attached Infiltration Test Results page. This rate should be considered preliminary and should be confirmed once the facility has been completed.

Test Location	Test Depth (inches) BGS	Infiltration Rate (in/hr)
IT-1	40	17.2
IT-2	48	12.0
IT-3	55	8.9

**Conclusion** 

The above infiltration rates indicate the finer grained soils have a lower rate of infiltration than the surficial coarser grained materials. The site infiltration rates are, in our experience, generally representative of the soil type(s) encountered and it is our opinion that on-site disposal of stormwater at the site is feasible. However, soil type and consistency likely varies throughout the site and soil conditions should be verified by the EOR. The area(s) proposed for infiltration shall not be subjected to compaction of the soil by vehicle traffic, storage of materials, or other means that can influence the rate of infiltration in those areas. The required setback from property lines and structures is 5-feet and 10-feet, respectively. It is the client/design professional's responsibility to determine that the placement of the infiltration basin meets these requirements.

#### **Limitations**

This report has been prepared for the exclusive use of the addressee and their designated representatives for the design of the proposed development. The analysis and recommendations contained herein were prepared in general accordance with the standards of practice for the area at the time of this report's preparation and may not be suitable for purposes other than those described in this report.

Subsurface explorations indicate soil conditions at specific locations and depths and do not necessarily reflect soil and groundwater variations that may exist between other locations at the site. If design changes are made that may affect the results of our testing, or a substantial amount of time passes between our investigation and the site development, we reserve the right to review the changes for applicability.

We assume no responsibility or liability for engineering, inspection, or testing performed by others and no warranty, expressed or implied, is given. Use of this report constitutes an agreement and consent by the addressee and their designated representatives to the limitations listed above. If you have any questions regarding the test method, data analysis or design, please contact BEI Staff at (503) 779-2577.

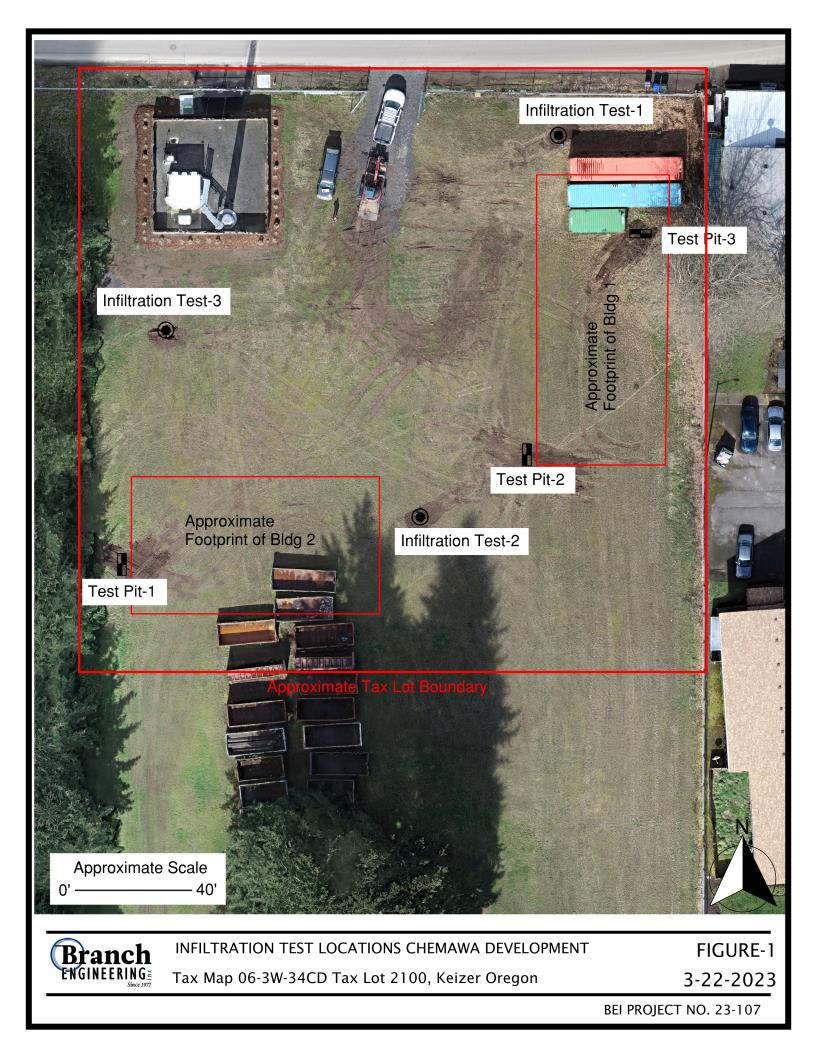
## Sincerely, *Branch Engineering Inc*,



Ronald J. Derrick, P.E., G.E. Principal Geotechnical Engineer

## Attached:

Figure-1 Site Map Infiltration Test Results (1) ORWD Well Logs (3) NRCS Soils Map



## **Infiltration Test Results**

Project:Chemawa Partition ApartmentsProject No:23-107Test Date:3/8/2023Test Type:Encased Falling HeadPipe Diameter:6 inches



Test No:	IT-1	Pipe Above Grade:	16 inches	Test Depth:40 inches	
Depth (inches)				Soil Description	
	0	12	Brown Silt with fine roots		
	12	36	Brown silty sand (SM)		
36 40			Brown	n silt with little sand (ML)	

## Trial 1 Time Start: 14:20

Elapsed Time (min)	Depth to water surface (in)	Depth of Water (in)	Drop in Water Level (in)	Infiltration Rate (in/hr)	Remarks
0	44.0	12			
5	46.0	10	2.0	24.0	
7	46.8	9.3	0.8	22.5	
14	49.3	6.8	2.5	21.4	
19	51.0	5.0	1.8	21.0	
27	53.8	2.3	2.8	20.6	
35	56	0.0	2.3	16.9	

Trial 2	<b>Time Start:</b> 14:56				
Elapsed Time (min)	Depth to water surface (in)	Depth of Water (in)	Drop in Water Level (in)	Infiltration Rate (in/hr)	Remarks
0	44.0	12			
5	45.8	10.3	1.8	21.0	
10	47.3	8.7	1.6	18.6	
20	50.3	5.8	3.0	17.7	
30	53.1	2.9	2.9	17.1	
40	56.0	0	2.9	17.4	

Trial 3	<b>Time Start:</b> 15:40				
Elapsed Time (min)	Depth to water surface (in)	Depth of Water (in)	Drop in Water Level (in)	Infiltration Rate (in/hr)	Remarks
0	44.5	11.5			
5	46.2	9.8	1.7	20.4	
10	47.8	8.2	1.6	19.2	
20	50.8	5.3	3.0	17.7	
30	53.6	2.4	2.9	17.1	
40	56.0	0	2.4	17.3	

## **Infiltration Test Results**

Project:Chemawa Partition ApartmentsProject No:23-107Test Date:3/8/2023Test Type:Encased Falling HeadPipe Diameter:6 inches



Test No:	IT-2	Pipe Above Grade:	0 inches	Test Depth:48 inches	
	Depth	(inches)		Soil Description	
	0	12	Bro	own Silt with fine roots	
	12	36	Brown silty sand (SM)		
36 48			Brown	n silt with little sand (ML)	

## Trial 1 Time Start: 14:20

Elapsed Time (min)	Depth to water surface (in)	Depth of Water (in)	Drop in Water Level (in)	Infiltration Rate (in/hr)	Remarks
0	37.0	11			
5	38.5	10	1.5	18.0	
7	39.0	9.0	0.5	15.0	
14	40.6	7.4	1.6	13.7	
19	41.8	6.3	1.2	13.8	
27	43.4	4.6	1.7	12.4	
45	47.0	1.0	3.6	12.0	

Trial 2	<b>Time Start:</b> 15:02				
Elapsed Time (min)	Depth to water surface (in)	Depth of Water (in)	Drop in Water Level (in)	Infiltration Rate (in/hr)	Remarks
0	36.5	11.5			
5	37.8	10.3	1.3	15.0	
10	38.9	9.1	1.2	13.8	
20	41.0	7.0	2.1	12.6	
30	43.0	5.0	2.0	12.0	
40	45.0	3.0	2.0	12.0	

Trial 3	<b>Time Start:</b> 15:44				
Elapsed Time (min)	Depth to water surface (in)	Depth of Water (in)	Drop in Water Level (in)	Infiltration Rate (in/hr)	Remarks
0	36.0	12			
5	37.2	10.8	1.2	14.4	
10	38.3	9.7	1.1	13.2	
20	40.5	7.5	2.2	13.2	
30	42.6	5.4	2.1	12.6	
40	44.6	3.4	2.0	12.0	
50	46.6	1.4	2	12.0	

## **Infiltration Test Results**

Project:Chemawa Partition ApartmentsProject No:23-107Test Date:3/8/2023Test Type:Encased Falling HeadPipe Diameter:6 inches



Test No:	IT-3	Pipe Above Grade:	0 inches	Test Depth: 55 inc	hes
	Depth	(inches)		Soil Description	
	0	12	Bro	wn Silt with fine roots	
	12	46	В	rown silty sand (SM)	
	46	55	Brown	n silt with little sand (ML)	

## Trial 1 Time Start: 14:25

Elapsed Time (min)	Depth to water surface (in)	Depth of Water (in)	Drop in Water Level (in)	Infiltration Rate (in/hr)	Remarks
0	45.0	10			
5	46.0	9	1.0	12.0	
10	46.9	8.1	0.9	10.8	
14	47.6	7.4	0.7	10.5	
20	48.6	6.4	1.0	10.0	
30	50.2	4.8	1.6	9.6	
55	54.0	1.0	3.8	9.1	

Trial 2	<b>Time Start:</b> 15:26				
Elapsed Time (min)	Depth to water surface (in)	Depth of Water (in)	Drop in Water Level (in)	Infiltration Rate (in/hr)	Remarks
0	44.0	11			
5	44.9	10.1	0.9	10.8	
10	45.7	9.3	0.8	9.6	
30	49.0	6.0	3.3	9.9	
40	50.5	4.5	1.5	9.0	
58	53.2	1.8	2.7	9.0	

Trial 3	<b>Time Start:</b> 16:30				
Elapsed Time (min)	Depth to water surface (in)	Depth of Water (in)	Drop in Water Level (in)	Infiltration Rate (in/hr)	Remarks
0	45.5	9.5			
10	47.2	7.8	1.7	10.2	
20	48.8	6.2	1.6	9.5	
30	50.3	4.7	1.5	9.1	
40	51.8	3.2	1.5	9.0	
55	54.0	1.0	2.2	8.8	

STATE OF OREGON		DLC.0619	389 MAR	5	s/31	$   \sqrt{z} $	33dc
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## STATE OF OREGON GEOTECHNICAL HOLE REPORT 5)

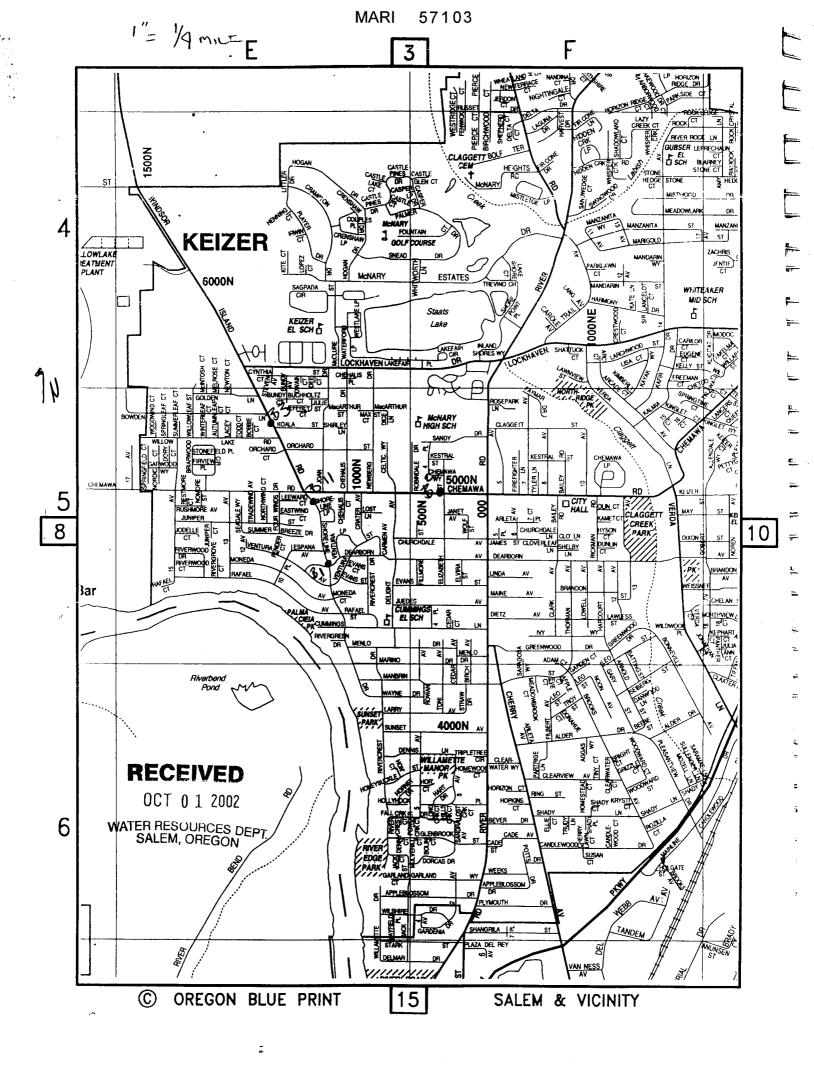
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THIS REPORT MUST BE SUBMITTED TO THE WATER RESOURCES DEPARTMENT WITHIN 30 DAYS OF COMPLETION OF WORK

ORIGINAL & FIRST COPY-WATER RESOURCES DEPARTMENT SECOND COPY-CONSTRUCTOR THIRD COPY-CUSTOMER



#### STATE OF OREGON GEOTECHNICAL HOLE REPORT (as required by OAR 690-240-0035)

**MARI 69916** 

6/15/2021

(1) OWNER/PROJECT Hole Number <u>B1</u>	-
PROJECT NAME/NBR: LOREN'S SANITATION	(9) LOCATION OF HOLE (legal description)
First Name     Last Name       Company     BLACK MOUNTAIN CONSULTING - OWNER'S REP       Address     22566 SW WASHINGTON ST., STE. 206	County       MARION       Twp       6.00       S       N/S       Range       3.00       W       E/W WM         Sec       34       SE       1/4 of the       SW       1/4       Tax Lot       2100         Tax Map Number
City     SHERWOOD     State     OR     Zip     97140	Long or -123.04464167 DMS or DD
(2) TYPE OF WORK X New Deepening Abandonment	O Street address of hole     Nearest address       1141 CHEMAWA RD KEIZER
(3) CONSTRUCTION         Rotary Air       Hand Auger         Rotary Mud       Cable         Other	(10) STATIC WATER LEVEL Date SWL(psi) + SWL(ft) Existing Well / Predeepening Completed Well
(4) TYPE OF HOLE:	WATER BEARING ZONES       Flowing Artesian?         Depth water was first found
Ouncased Temporary Cased Permanent     Ouncased Permanent     Other     Other:	SWL Date     From     To     Est Flow     SWL(psi)     +     SWL(ft)
(5) USE OF HOLE	(11) SUBSURFACE LOG Ground Elevation
GEOTECHNICAL SOIL	Material     From     To       Topsoil     0     2       Brown Soft Silt     2     23       Small Gravels/Sands/Cobbles     23     40
(6) BORE HOLE CONSTRUCTION       Special Standard       Attach copy         Depth of Completed Hole       40.00       ft.         BORE HOLE       SEAL       sacks/         Dia       From       To       Material       From       To       Amt       lbs         8.25       0       40       Bentonite Chips       0       40       21       S         Backfill placed from	Date Started <u>6/9/2021</u> Completed <u>6/9/2021</u> (12) ABANDONMENT LOG: sacks/
(7) CASING/SCREEN	- Material From To Amt Ibs Bentonite Chips 0 40 21 S
Casing Screen Dia + From To Gauge Stl Plstc Wld Thrd	
(8) WELL TESTS	Date Started 6/9/2021 Completed 6/9/2021
Pump       Bailer       Air       Flowing Artesian         Yield gal/min       Drawdown       Drill stem/Pump depth       Duration(hr)         Image: Supervising Geologist/Engineer       °F Lab analysis       Yes       By         Supervising Geologist/Engineer       Water quality concerns?       Yes (describe below) TDS amount       Image: Supervising Geologist/Engineer	Date Started 6/9/2021       Completed 6/9/2021         Professional Certification (to be signed by an Oregon licensed water or monitoring well constructor, Oregon registered geologist or professional engineer).         I accept responsibility for the construction, deepening, alteration, or abandonment work performed during the construction dates reported above. All work performed during this time is in compliance with Oregon geotechnical hole construction standards. This report is true to the best of my knowledge and belief.
From To Description Amount Units	License/Registration Number <u>1772</u> Date <u>6/15/2021</u>
	First Name       WILLIAM       Last Name       WRIGHT         Affiliation       WESTERN STATES SOIL CONSERVATION, INC.

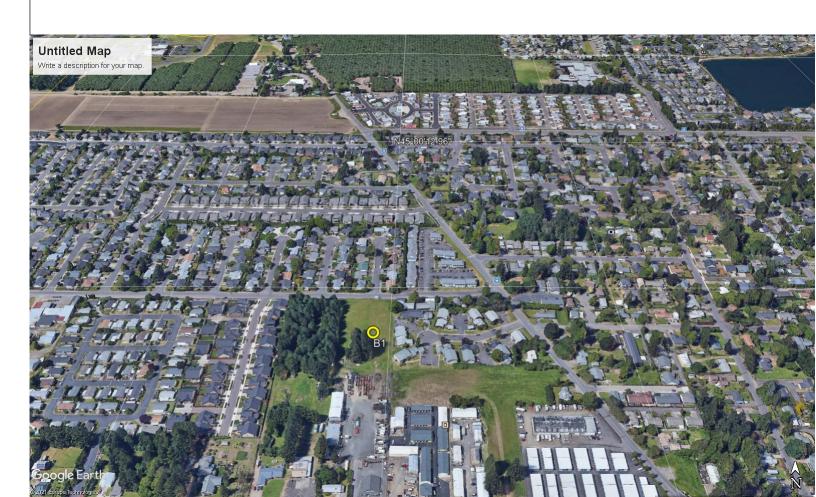
**ORIGINAL - WATER RESOURCES DEPARTMENT** 

ORIGINAL - WATER RESOURCES DEPARTMENT WITHIN 30 DAYS OF COMPLETION OF WORK THIS REPORT MUST BE SUBMITTED TO THE WATER RESOURCES DEPARTMENT WITHIN 30 DAYS OF COMPLETION OF WORK Form Version:

GEOTECHNICAL HOLE REPORT - Map with location identified must be attached and shall include an approximate scale and north arrow

6/15/2021

## Map of Hole

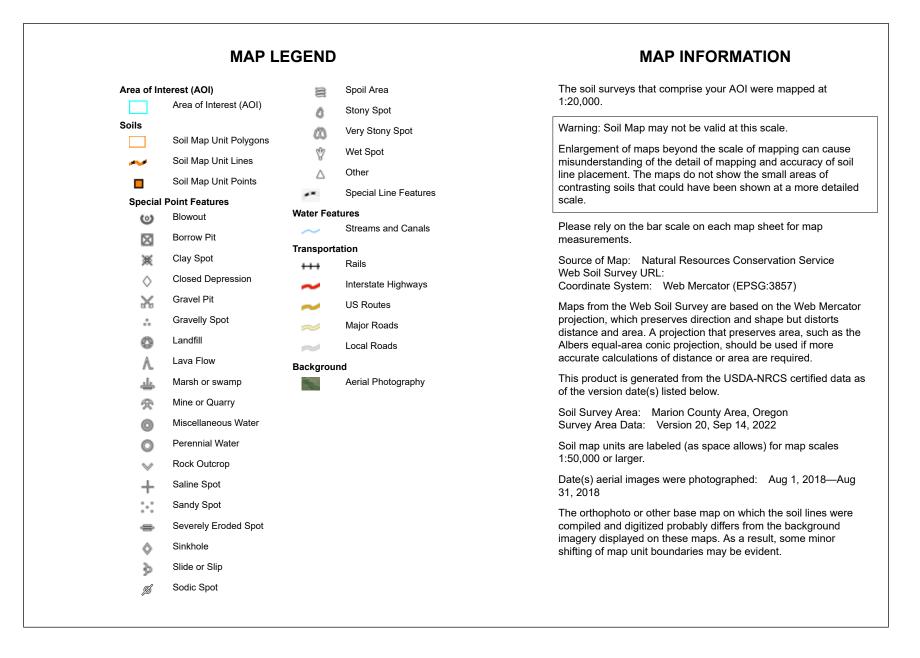




Page 1 of 3

Conservation Service

Web Soil Survey National Cooperative Soil Survey





## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Cm	Cloquato silt loam	30.4	54.5%
Nu	Newberg fine sandy loam	5.4	9.7%
Nw	Newberg silt loam	20.0	35.9%
Totals for Area of Interest		55.9	100.0%





United States Department of Agriculture

Natural Resources Conservation

Service

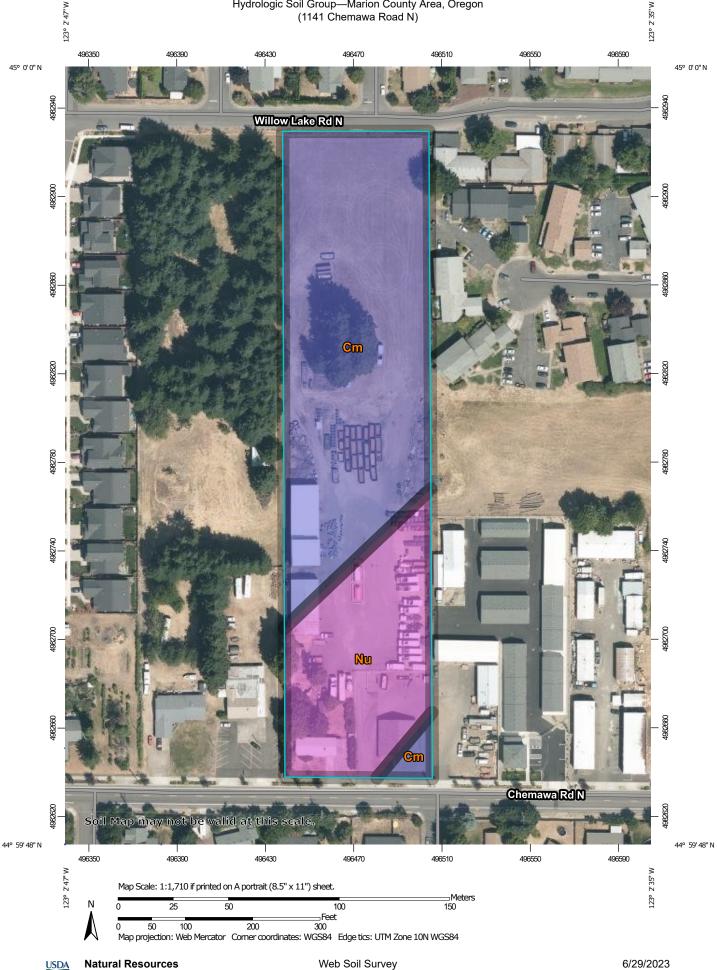
A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

## Custom Soil Resource Report for Marion County Area, Oregon

1141 Chemawa Road N

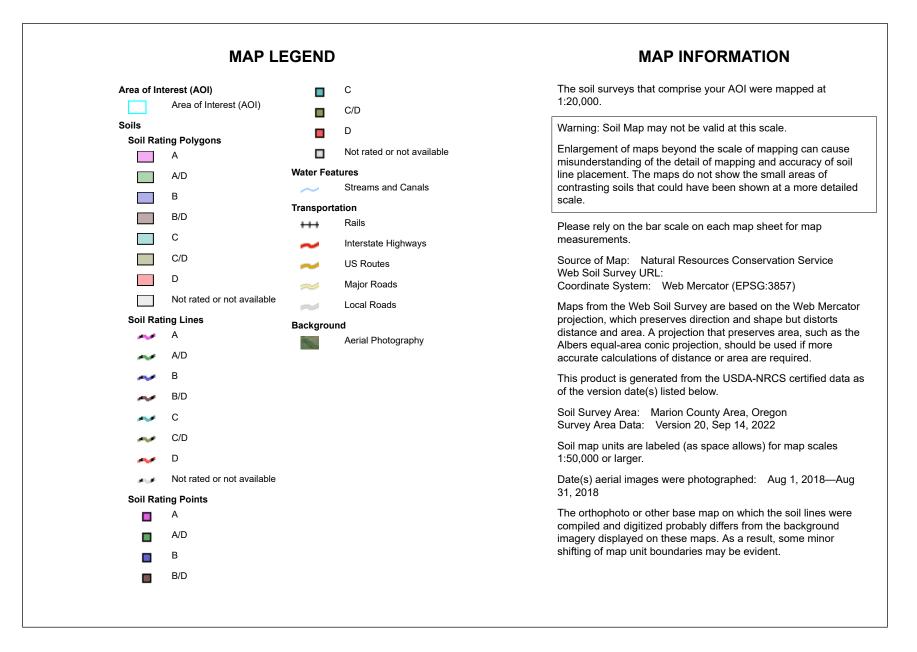


#### Hydrologic Soil Group—Marion County Area, Oregon (1141 Chemawa Road N)



National Cooperative Soil Survey

**Conservation Service** 





## Hydrologic Soil Group

Map unit symbol	Map unit symbol Map unit name		unit symbol Map unit name Rati		Acres in AOI	Percent of AOI
Cm	Cloquato silt loam	В	3.3	67.9%		
Nu	Newberg fine sandy loam	A	1.5	32.1%		
Totals for Area of Intere	st		4.8	100.0%		

## Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

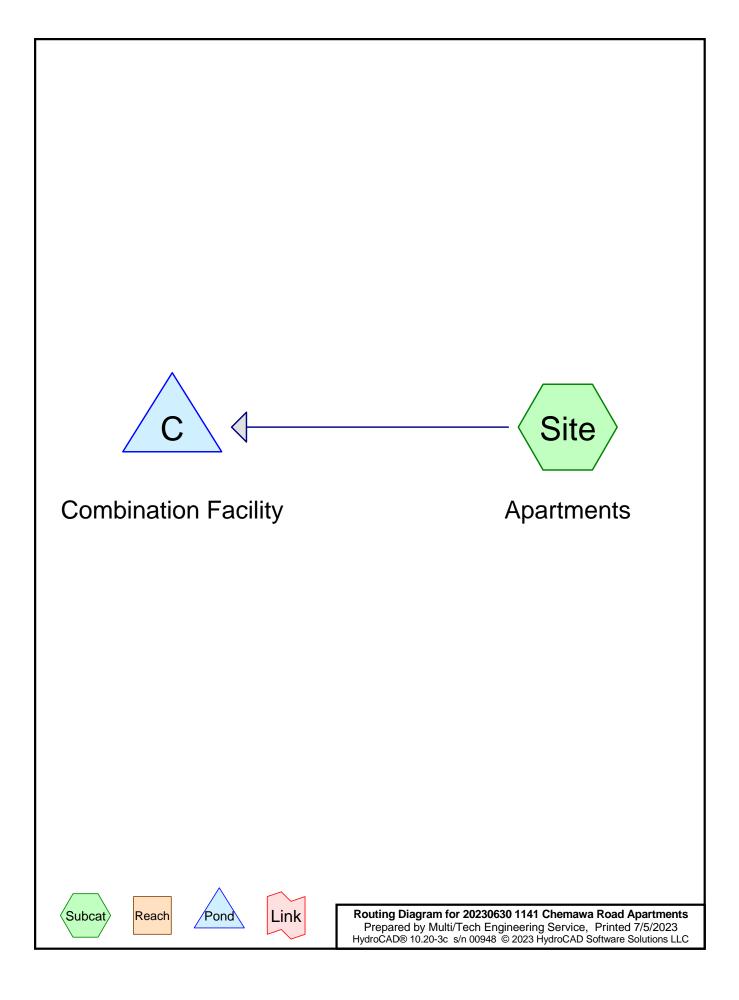
If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

## **Rating Options**

Aggregation Method: Dominant Condition

USDA

Appendix C



HydroCAD® 10.20-3c s/n 00948 © 2023 HydroCAD Software Solutions LLC

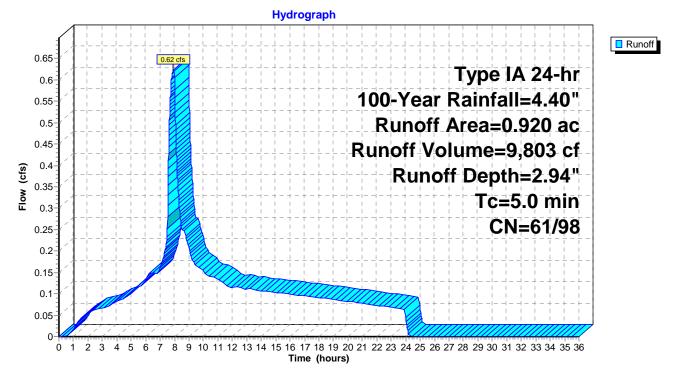
## Summary for Subcatchment Site: Apartments

Runoff = 0.62 cfs @ 7.91 hrs, Volume= 9,803 cf, Depth= 2.94" Routed to Pond C : Combination Facility

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type IA 24-hr 100-Year Rainfall=4.40"

	Area	(ac)	CN	Desc	cription						
*	0.	560	98	Impe	Impervious surface, HSG B						
	0.	360	61	>75%	>75% Grass cover, Good, HSG B						
	0.	920	84	Weig	ghted Aver	age					
	0.	360	61	39.1	3% Pervio	us Area					
	0.	560	98	60.8	7% Imperv	vious Area					
	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	5.0						Direct Entry, Assumed				

## **Subcatchment Site: Apartments**



HydroCAD® 10.20-3c s/n 00948 © 2023 HydroCAD Software Solutions LLC

## **Summary for Pond C: Combination Facility**

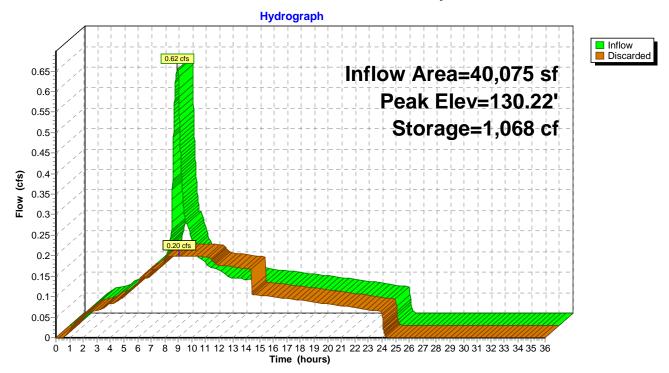
Inflow Area =	40,075 sf, 60	0.87% Impervious,	Inflow Depth = 2.94"	for 100-Year event
Inflow =	0.62 cfs @ 7	7.91 hrs, Volume=	9,803 cf	
Outflow =	0.20 cfs @ 9	9.06 hrs, Volume=	9,803 cf, Atter	n= 68%, Lag= 69.0 min
Discarded =	0.20 cfs @ 9	9.06 hrs, Volume=	9,803 cf	

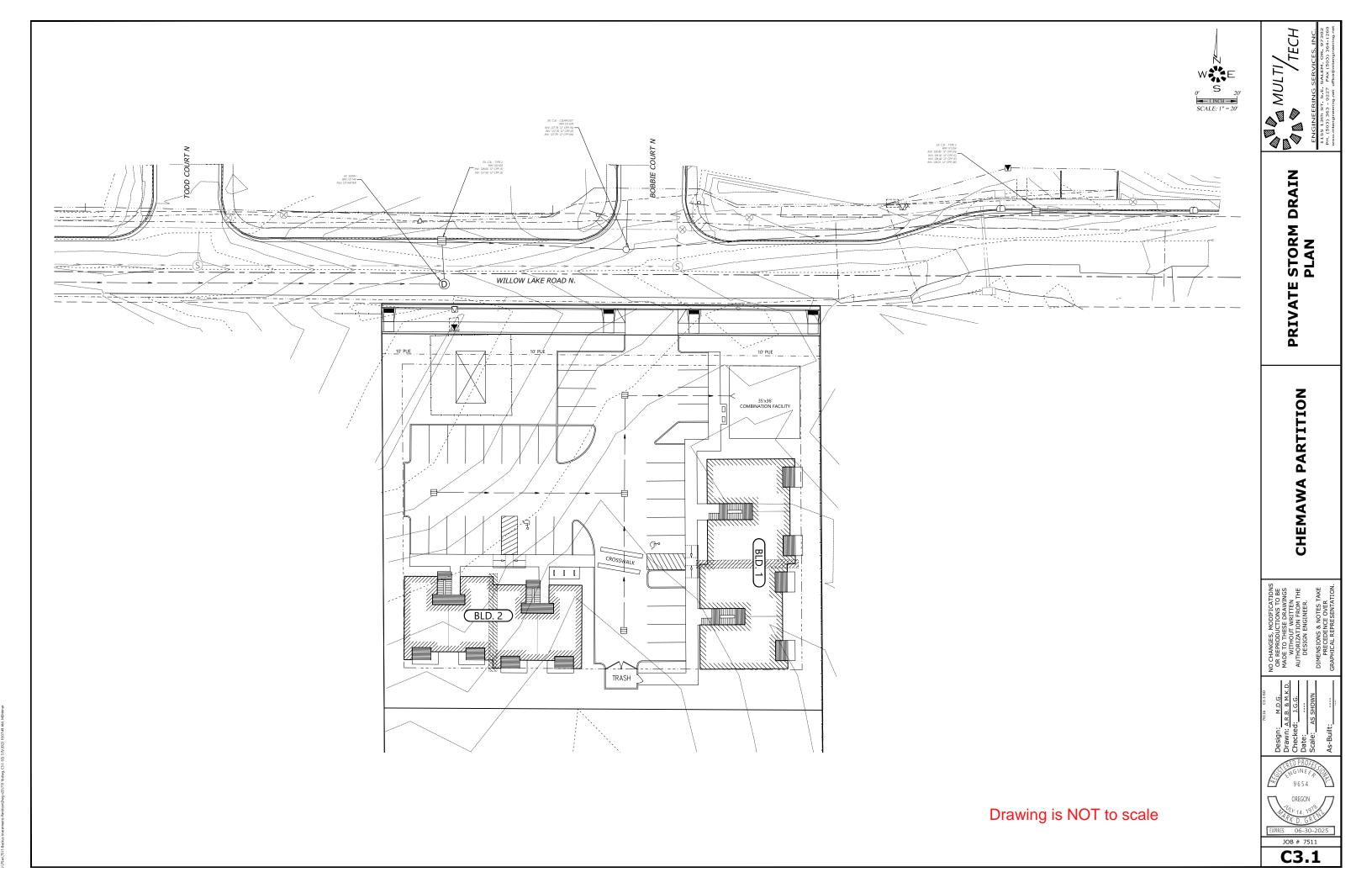
Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Peak Elev= 130.22' @ 9.06 hrs Surf.Area= 1,600 sf Storage= 1,068 cf

Plug-Flow detention time= 26.8 min calculated for 9,803 cf (100% of inflow) Center-of-Mass det. time= 26.8 min (718.3 - 691.5)

Volume	Invert Ava	il.Storage	Storage Descrip	tion	
#1	127.24'	3,916 cf	Custom Stage Data (Prismatic) Listed below (Recalc)		ed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
127.24	1,600	0.0	0	0	
127.25	1,600	35.0	6	6	
128.24	1,600	35.0	554	560	
128.25	1,600	5.0	1	561	
129.99	1,600	5.0	139	700	
130.00	1,600	100.0	16	716	
132.00	1,600	100.0	3,200	3,916	
	3	7.24' <b>4.50</b>		<b>n over Horizontal a</b> dwater Elevation = 1	

**Discarded OutFlow** Max=0.20 cfs @ 9.06 hrs HW=130.22' (Free Discharge) **1=Exfiltration** (Controls 0.20 cfs) **Pond C: Combination Facility** 





Appendix D

## Matt Hendrick, P.E.

From: Sent: To: Cc: Subject: Waldner, Haley <WaldnerH@keizer.org> Thursday, June 29, 2023 3:07 PM Matt Hendrick, P.E. Jeremy Grenz; Blaylock, Keare RE: Water Depth

Matt,

Here is what I found for groundwater depth at 1141 Chemawa road N.

Depth to water: between 25-28ft

\*This is data collected from a groundwater model and may not be completely accurate\*

Thank you,

#### **Haley Waldner**

Environmental Compliance Coordinator City Of Keizer, Public Works Office: (503)856-3424 Mobile: (503)932-5040

From: Matt Hendrick, P.E. <MHendrick@mtengineering.net>
Sent: Thursday, June 29, 2023 2:15 PM
To: Waldner, Haley <WaldnerH@keizer.org>
Cc: Jeremy Grenz <JGrenz@mtengineering.net>
Subject: Water Depth

# CAUTION: This email originated from Outside Your Organization. Exercise caution when opening attachments or on clicking links from unknown senders. Please <u>contact Information Technology</u> <u>for assistance.</u>

Haley,

Could you please give me that water depth at 1141 Chemawa Road N. We are in the process of designing a UIC system for a small apartment complex located at the address. I have attached a couple maps for reference.

Matt

Matthew W. Hendrick, P.E. (OR, WA, ID, AK, AZ, UT) Civil Engineer

Multi/Tech Engineering Services, Inc. 1155 13th Street S.E. Salem, OR 97302 Office: (503) 363-9227

## STANDARD O&M PLAN FOR THE SIMPLIFIED APPROACH

## **Rain Gardens**

Structural components must be operated and maintained in accordance with the design specifications.					
MAINTENANCE INDICATOR	CORRECTIVE ACTION				
Clogged gutters, drains, downspouts, or inlets	Remove sediment, debris, and blockages from downspouts, gutters, pipes, and inlets to maintain at least 50% conveyance at all times. Clean at least twice a year depending on the presence of overhanging trees. Clear any build-up of soil, bark dust, and/or vegetative growth from around downspout extension and/or splash blocks. Verify there is sufficient slope so that water flows away from the foundation.				
Damaged or missing pipes,	Repair or replace broken gutters and downspouts as needed. Identify possible leaks and verity				
gutters, and downspouts	that roof flashing directs water into gutters. Look for low spots or sagging areas along the				
gutter line and repair as needed with new hangers.					
Vegetation must cover at least 90	0% of the facility at maturity.				
MAINTENANCE INDICATOR	CORRECTIVE ACTION				
Dead or stressed vegetation	Remove dead material; replant per original planting plan, or substitute from the plant list in				
	Section 3.5.				
Dry grass or other plants	Irrigate and mulch as needed. Maintain grass height at 6"-9".				
Weeds	Manually remove weeds				
Growing medium must sustain healthy plant cover and drain within 48 hours.					
MAINTENANCE INDICATOR	CORRECTIVE ACTION				
Gullies, erosion, exposed	Fill in and lightly compact areas of erosion with City-approved soil mix (see SWMM section				
soils, sediment	3.2.2.1) and replant according to planting plan or substitute from the plant list in SWMM				
accumulation	section 3.8. Any erosion deeper than 2 inches must be addressed. Sediment more than 4				
	inches deep must be removed.				
Scouring at the inlet(s)	Ensure splash blocks or inlet gravel/rock are placed correctly to prevent erosion.				
Ponding	Till, amend, or rake soil as needed to ensure ponding water drains within 48 hours.				

## **Annual Maintenance Schedule**

Summer	Make structural repairs; clean gutters and downspouts; remove any build-up of weeds or organic debris.
Fall	Replant exposed soil and replace dead plants. Remove sediment and plant debris.
Winter	Clear gutters and downspouts.
Spring	Remove sediment and plant debris. Replant exposed soil and replace dead plants.
All seasons	Weed as necessary.

- Maintenance Records: All facility operators are required to keep an inspection and maintenance log. Record date, description, and contractor (if applicable) for all repairs, landscape maintenance, and facility cleanout activities. Keep work orders and invoices on file and make available upon request of the City inspector.
- **Fertilizers:** Their use is strongly discouraged because of the potential for negative environmental impacts. Never apply fertilizer before testing the fertility of the growing medium to determine whether fertilizer is needed and appropriate application rates. Use only organic, slow-release fertilizers. See SWMM Section 3.2.2.1 for more information.

Pesticides/Herbicides: Their use is prohibited.

**Pollution Prevention:** All sites must implement Best Management Practices to prevent the introduction of pollutants to stormwater and/or facility discharge points. In the event of a spill, call 503-823-7180 to report it immediately and document the circumstances and the corrective action taken; include the date/time, weather and site conditions. Never wash spills into a stormwater facility.

Infiltration/Flow Control: All facilities must drain within 48 hours. Record time/date, weather, and conditions when ponding occurs.

Vectors (Mosquitoes and Rats): Stormwater facilities must not harbor mosquito larvae or rodents that pose a threat to public health or that undermine the facility structure. Record the time/date, weather, and site conditions when vector activity observed. Record when vector abatement started and ended.

Access: Maintain ingress/egress per design standards, maintaining access to the entirety of the facility for inspection & maintenance.

## Operations and Maintenance Log

Date	Work Performed By	Type of Work Performed				ormed	Notes	Initials
	renormed by	Clean inlets and Outlets	Sediment and Trash Removal	Plant Replacement type, location	Structural Repairs – type, location	Other		